



<b>Job No.</b>	K19U0654_R2	<b>Designer</b>	AH
<b>Customer</b>	ALPHA RIDGE - ORGANICS	<b>Checker</b>	
<b>Address</b>	2350 MARIOTTSVILLE, RD	<b>Seal Engr.</b>	
	COLUMBIA, MD 21046	<b>Revision #</b>	

2

### Seismic Forces

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#### Roof Bracing

$R = 3.00$ ,  $Rho = 1.00$   
 $Cs = 0.0569$   
 $W = 25.90$  (k)  
 Force,  $V = 1.47$  (k)  
 Force,  $E = 1.47$  (k)

#### Sidewall Panel

Back  $R = 3.00$ ,  $Rho = 1.00$ ,  $Omega = 1.00$   
 $Cs = 0.0569$   
 $W = 0.00$  (k)  
 Force,  $V = 0.00$  (k)  
 Force,  $Em = 0.00$  (k)

#### Wind Bents

Front  $R = 3.00$ ,  $Rho = 1.00$   
 $Cs = 0.0569$   
 $W = 25.90$  (k)  
 Force,  $V = 1.47$  (k)  
 Force,  $E = 1.47$  (k)

#### Rigid Frames

$R = 3.00$ ,  $Rho = 1.00$   
 $Cs = 0.0569$   
 Frame 1  $W = 6.85$  (k)  
 Force,  $V = 0.39$  (k)  
 Force,  $E = 0.39$  (k)  
 Frame 2  $W = 6.68$  (k)  
 Force,  $V = 0.38$  (k)  
 Force,  $E = 0.38$  (k)  
 Frame 3  $W = 13.36$  (k)  
 Force,  $V = 0.76$  (k)  
 Force,  $E = 0.76$  (k)

#### End Plates

Frame  $R = 3.00$ ,  $Rho = 1.00$ ,  $Omega = 1.00$   
 Wind Bent  $R = 3.00$ ,  $Rho = 1.00$ ,  $Omega = 1.00$

#### Total Base Shear

Longitudinal  
 Force,  $V = 1.47$  (k)  
 Transverse  
 Force,  $V = 2.30$  (k)





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## GUTTER CALCULATOR

Job Number:	K13U0659
Engineer:	AH
Date:	10/16/2018
Version:	2.3

Denotes Input Cell

<b>Width of Drainage Area:</b>	40	(ft) Eave to Ridge Distance - Include all widths that drain to this gutter including existing (e) existing building draining onto new below-eave structure
<b>Rainfall Intensity:</b>	7.39	(in/hr) 5 Min Duration - 5-Yr Recurrence (MBMA I-1) for External Eave Gutters (in/hr) 5 Min Duration - 25-Yr Recurrence (MBMA I-2) for Internal, Parapet & Fascia Gutters
<b>Building Bay Spacing:</b>	40	Largest bay spacing down length of building - or user can check multiple bay spacings It is preferable to keep the gutter downspouts at the column lines for simplicity and visual
<b>Building Eave Line Gutter:</b>	KLS-RCM-92387 Eave Gutter Standard	Choose gutter type based on Order Documents - only upsize gutter as last option Basic guidance on special size gutter is to keep to standard width of 7.25"
<b>Building Eave Downspout:</b>	Corrugated Downspout Standard	Choose downspout size based on Order Documents - only upsize downspout as last option Basic guidance on special size gutter is to keep to standard depth of 6"

Special Size Gutter Width		Special Size Downspout Width		Special Size Downspout Depth	
Special Size Gutter Height		Special Size Downspout Width		Special Size Downspout Depth	
<b>Gutter Width:</b>	7.25 in	<b>Downspout Width:</b>	3.50 in	<b>Gutter Area:</b>	45.31 sq in
<b>Gutter Depth:</b>	6.25 in	<b>Downspout Depth:</b>	5.00 in	<b>Downspout Area:</b>	17.50 sq in

<u>Length A</u>	<u>Length B</u>	<u>Area C</u>
<b>Maximum spacing of gutter downspouts along eave:</b>	63' - 0"	See notation 'Length A' above
<b>Maximum spacing from end of gutter to center of first downspout:</b>	31' - 6"	See notation 'Length B' above
<b>Required downspout cross sectional area (40' - 0" Downspout Spacing):</b>	10 Sq-In	See notation 'Area C' above
		<b>Downspout Size OK</b>

Project No. : K19U0654  
 Description : WIND LOADS  
 Engineer : AH  
 Date : 10/22/2019

Wind Loading per ASCE 7-10

**Geometry**

(with AISI 2012 Specification and 2012 MBMA Manual.)

Version: 2019.05.23 (Date: 05/23/19) By NBG-GS

Building Name: Building A Building Type: Single-Slope Roof: Standing Seam 360

Bldg. Width [B]: 40.0000' NBG Brand: KBSTN

Bldg. Length [D]: 80.0000'

Left Eave Ht. [LEH]: 10.0000'

Right Eave Ht. [REH]: 16.6667'

Left Roof Slope: 2.00:12

Bay Width [Bay]: 20.0000'

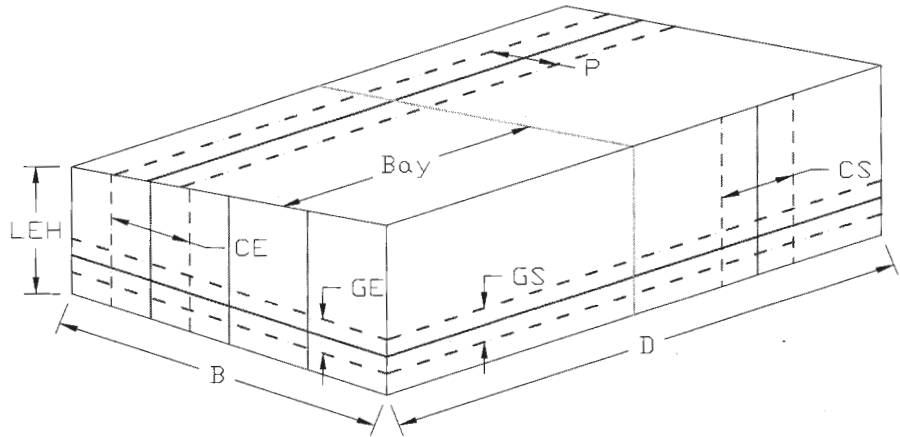
Purlin Trib. Width [P]: 5.0000'

EW Girt Trib. Ht. [GE]: 5.0000'

SW Girt Trib. Ht. [GS]: 5.0000'

EW Girt Length: 25.0000'

SW Girt Length: 20.0000'



EW Col. Trib. Width [CE]: 25.0000'

SW Col. Trib. Width [CS]: 20.0000'

Left SW Top-of-Parapet:   
 Opening Area:

Right SW Top-of-Parapet:   
 EW Top-of-Parapet:

**Loading Information**

Building Code: IBC 2015

Wind Speed: 115 mph

Wind Exposure: C

Building Porosity: Open - Obstructed Wind Flow

Interior Partition Walls? No

No. of Frames: 5

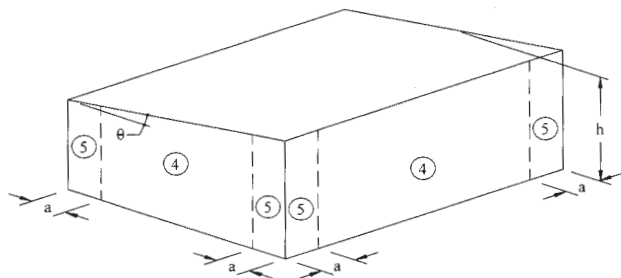
Eff. Solid Area: 73.4 sf

**General Loading Calculations**

h: 10.0000'  $K_d$ : 0.85  $K_{zt}$ : 1.00  $R_f$ : 1.00  $q_h$ : 24.43 psf

$K_z$  or  $K_h$ : 0.85  $G$ : 0.85  $GC_{pi}$ : 0.00

**Components and Cladding, Walls**



a = ----

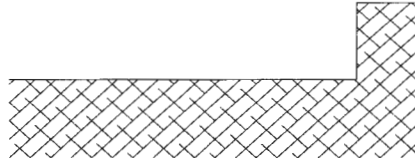
Item	Effective Area (ft <sup>2</sup> )	Pressure Zones 4,5 (psf)	Suction Zone 4 (psf)	Suction Zone 5 (psf)
Sidewall Wind Column	200	18.32	-18.32	-18.32
Endwall Wind Column	250	18.32	-18.32	-18.32
Sidewall Girt	133	18.32	-18.32	-18.32
Endwall Girt	208	18.32	-18.32	-18.32
Wall Panel	8	18.32	-18.32	-18.32

Note: Value of  $GC_{pi}$  in results above reduced by 10% per Note 5 of Figure 30.4-1 since slope angle is  $\leq 10^\circ$ .

### Wind Loading Continued...

#### Wall Parapet Structural

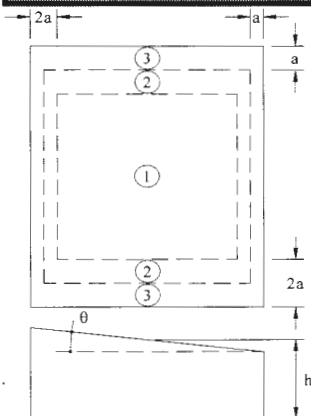
Case 1: Windward Total Load   
 Case 2: Leeward Total Load



Item	Maximum Projection (ft)	$K_{h\_par}$	$q_p$ (psf)	Windward Total Load (psf)	Leeward Total Load (psf)
FSW Parapet	---	---	---	---	---
BSW Parapet	---	---	---	---	---
EW Parapet	---	---	---	---	---

#### Components and Cladding, Roofs

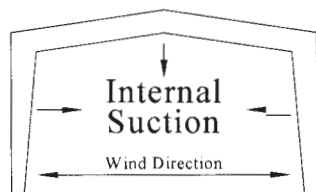
Applicable Roof Slope Angle = 9.46 deg  
 $a = 4.00$  ft.



Item	Effective Area (ft <sup>2</sup> )	Pressure in Zones			Suction in Zones		
		1 (psf)	2 (psf)	3 (psf)	1 (psf)	2 (psf)	3 (psf)
Purlin/Joist	133	18.78	18.78	18.78	-37.47	-37.47	-37.47
Panel	10	18.78	28.18	37.57	-37.47	-57.25	-101.01
Fastener	10	18.78	28.18	37.57	-37.47	-57.25	-101.01
<i>Values Below are for Overhang Portion of Roof</i>							
Purlin/Joist	133	18.78	18.78	18.78	-37.47	-37.47	-37.47
Panel	10	18.78	28.18	37.57	-37.47	-57.25	-101.01
Fastener	10	18.78	28.18	37.57	-37.47	-57.25	-101.01

#### Main Wind Force Resisting Systems (Transverse Wind Direction)

Applicable Roof Slope Angle = 9.46 deg  
 $a = ---$



**WIND W1R W1L  
W3R W3L**

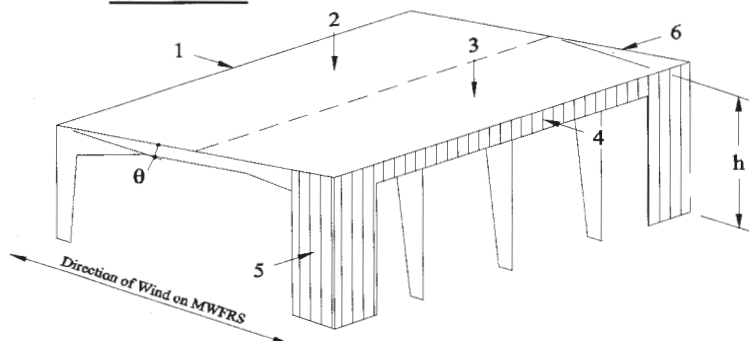


**WIND W2R W2L  
W4R W4L**

Item	Transverse Wind Direction							
	W1R	W1L	W2R	W2L	W3R	W3L	W4R	W4L
C1:	0.75	-0.75	0.75	-0.75	----	----	----	----
Load, (psf)	18.32	-18.32	18.32	-18.32	----	----	----	----
C2:	-0.04	-1.28	0.77	-0.64	----	----	----	----
Load, (psf)	-0.89	-31.15	18.78	-15.53	----	----	----	----
C3:	-1.00	-0.87	-0.26	-1.53	----	----	----	----
Load, (psf)	-24.37	-21.31	-6.23	-37.47	----	----	----	----
C4:	-0.75	0.75	-0.75	0.75	----	----	----	----
Load, (psf)	-18.32	18.32	-18.32	18.32	----	----	----	----
C5:	-0.75	-0.75	-0.75	-0.75	----	----	----	----
Load, (psf)	-18.32	-18.32	-18.32	-18.32	----	----	----	----
C6:	-0.75	-0.75	-0.75	-0.75	----	----	----	----
Load, (psf)	-18.32	-18.32	-18.32	-18.32	----	----	----	----

**Edge Zone Pressure Coefficients**

Item	Edge Zone Pressure Coefficients			
	W1R & W3R	W1L & W3L	W2R & W4R	W2L & W4L
C1E:	----	----	----	----
Load, (psf)	----	----	----	----
C2E:	----	----	----	----
Load, (psf)	----	----	----	----
C3E:	----	----	----	----
Load, (psf)	----	----	----	----
C4E:	----	----	----	----
Load, (psf)	----	----	----	----

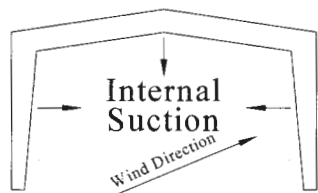


Wind Loading Continued...

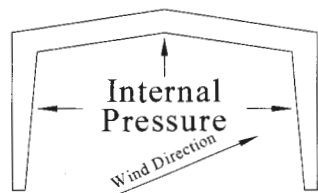
**Main Wind Force Resisting Systems (Longitudinal Wind Direction)**

Applicable Roof Slope Angle = 9.46 deg

a = 4.00 ft.

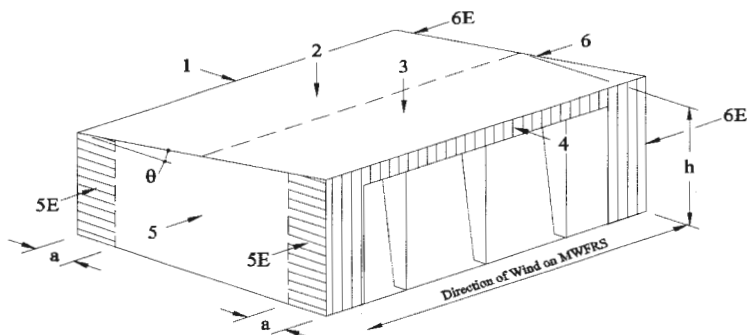


**WIND W5B W5F  
W7B W7F**



**WIND W6B W6F  
W8B W8F**

Item	Longitudinal Wind Direction							
	W5B	W5F	W6B	W6F	W7B	W7F	W8B	W8F
C1:	-0.75	-0.75	-0.75	-0.75	----	----	----	----
Load, (psf)	-18.32	-18.32	-18.32	-18.32	----	----	----	----
C2:	-1.02	-0.51	0.43	0.26	----	----	----	----
Load, (psf)	-24.92	-12.46	10.38	6.23	----	----	----	----
C3:	-1.02	-0.51	0.43	0.26	----	----	----	----
Load, (psf)	-24.92	-12.46	10.38	6.23	----	----	----	----
C4:	-0.75	-0.75	-0.75	-0.75	----	----	----	----
Load, (psf)	-18.32	-18.32	-18.32	-18.32	----	----	----	----
C1E:	----	----	----	----	----	----	----	----
Load, (psf)	----	----	----	----	----	----	----	----
C2E:	----	----	----	----	----	----	----	----
Load, (psf)	----	----	----	----	----	----	----	----
C3E:	----	----	----	----	----	----	----	----
Load, (psf)	----	----	----	----	----	----	----	----
C4E:	----	----	----	----	----	----	----	----
Load, (psf)	----	----	----	----	----	----	----	----



Item	End-Wall Pressure Coefficients			
	W5B & W7B	W5F & W7F	W6B & W8B	W6F & W8F
C5:	0.47	-0.34	0.47	-0.34
Load, (psf)	11.46	-8.31	11.46	-8.31
C6:	-0.34	0.47	-0.34	0.47
Load, (psf)	-8.31	11.46	-8.31	11.46
C5E:	0.72	-0.50	0.72	-0.50
Load, (psf)	17.48	-12.32	17.48	-12.32
C6E:	-0.50	0.72	-0.50	0.72
Load, (psf)	-12.32	17.48	-12.32	17.48

Wind Uplift for Bracing Input: -14.79 psf  
 Longitudinal Force Resisted by Bracing: 11.61 kip

Total Longitudinal Net Pressure Applied to Building: 21.77 psf  
 Total Longitudinal Force Applied to Building: 11.61 kip



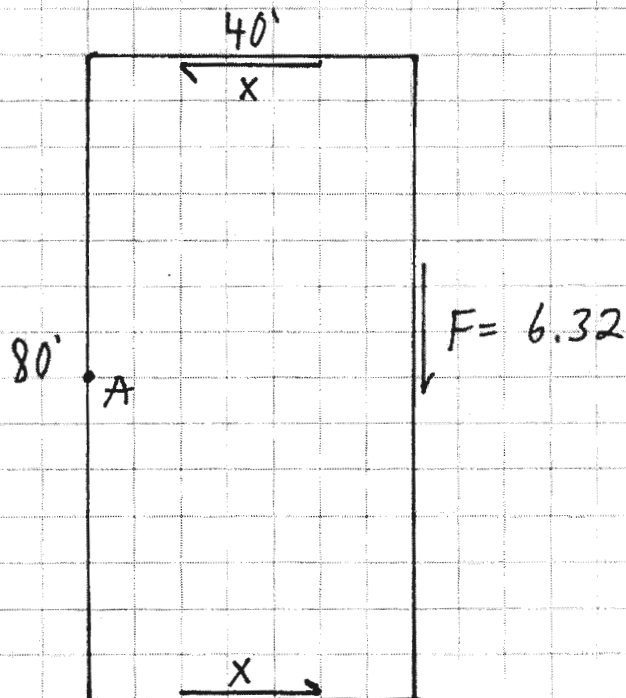
Racking Force Calcs.

BY *ATT*

CHK'D

JOB NO. *K19U0654*


SHT NO.



$$\Sigma M_A = 0 = (6.32 \cdot 40) - (x \cdot 80) \Rightarrow x = 3.2$$

*3.2 kips added to WLong loads on end frames.*



 <b>KIRBY BUILDING SYSTEMS</b> <small>A LUCOR Company</small>		<b>Building Enclosure Classification Check</b>		Job Num:	K19U0654
				Engineer:	AH
				Date:	10/16/2019
				Version:	1.4
<b>Building Information:</b>					
Building Length:	80		Back E.H.:	16	
Building Width:	40		Front E.H.:	22.6667	
Peak Offset:	40		Back Roof Slope:		XX/12 Slope
Gable/SS:	Single Slope		Front Roof Slope:		XX/12 Slope
<b>Building Area and Openings Information:</b>					
	Ag- Gross Area	Ag- Override	Ao - Open Area	Aoi	Agi
Back Roof:	3200.00			3413.34	4373.34
Front Roof:	0.00			3413.34	7573.34
Back Sidewall:	1280.00		800.00	2613.34	6293.34
Front Sidewall:	1813.34		1813.34	1600.00	5760.00
Left Endwall:	640.00		400.00	3013.34	6933.34
Right Endwall:	640.00		400.00	3013.34	6933.34
Auxiliary Wall 1:				3413.34	7573.34
Auxiliary Wall 2:				3413.34	7573.34
Auxiliary Roof 1:				3413.34	7573.34
Auxiliary Roof 2:				3413.34	7573.34
<b>Partially Enclosed Building Requirements: Not Satisfied</b>					
	Ao > 4 sq ft or Ao > 0.01Ag & Aoi/Agi ≤ 20%		Ao > 4 sq ft or Ao > 0.01Ag & Aoi/Agi ≤ 20%		
	Ao > 1.10Aoi		Ao > 1.10Aoi		
Back Sidewall:	Not Satisfied	Not Satisfied	Auxiliary Wall 1:	Not Satisfied	Not Satisfied
Front Sidewall:	Satisfied	Not Satisfied	Auxiliary Wall 2:	Not Satisfied	Not Satisfied
Left Endwall:	Not Satisfied	Not Satisfied	Auxiliary Roof 1:	Not Satisfied	Not Satisfied
Right Endwall:	Not Satisfied	Not Satisfied	Auxiliary Roof 2:	Not Satisfied	Not Satisfied
<b>Open Building Requirements: Not Satisfied</b>					
	% Open	Ao ≥ 0.8Ag	% Open	Ao ≥ 0.8Ag	
Back Sidewall:	62.50%	Not Satisfied	Auxiliary Wall 1:	100.00%	Satisfied
Front Sidewall:	100.00%	Satisfied	Auxiliary Wall 2:	100.00%	Satisfied
Left Endwall:	62.50%	Not Satisfied			
Right Endwall:	62.50%	Not Satisfied			
<b>Enclosed Building Requirements: Satisfied</b>					
Partially Enclosed Building Requirements:		Not Satisfied			
Open Building Requirements:		Not Satisfied			
<b>Classification:</b>	<b><u>Enclosed</u></b>		<b>Consideration has been made for wind openings in the design of this structure.</b>		
ASCE 7 -					
<b>Partially Enclosed Building:</b> A building that complies with both of the following conditions:					
The total area of openings in a wall that receives positive external pressure exceeds the sum of the areas of openings in the balance of the building envelope (walls and roof) by more than 10 percent.					
The total area of openings in a wall that receives positive external pressure exceeds 4 sq ft or 1 percent of the area of that wall, whichever is smaller, and the percentage of openings in the balance of the building envelope does not exceed 20 percent.					
<b>Open Building:</b> A building having each wall at least 80 percent open.					
<b>Enclosed Building:</b> A building that does not comply with the requirement for open or partially enclosed buildings.					



**A Nucor Company**

124 KIRBY DRIVE, PORTLAND, TN 37148



METAL BUILDING MANUFACTURERS ASSOCIATION  
MEMBER



ACCREDITED  
Metal Building Systems  
AC 472

JOB NUMBER: K19U0654A  
BUYER: MTD ERECTORS, INC.  
PROJECT: ALPHA RIDGE - ORGANICS STORAGE  
LOCATION: COLUMBIA, MD 21046

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E4	CROSS SECTION AT LINES 3
E5	ROOF FRAMING PLAN
E6	ROOF SHEETING PLAN
E7	SIDEWALL FRAMING LINES: A, B
E8	SPECIAL DETAILS

RECEIVED  
MAR 13 2020  
LICENSES & PERMITS  
DIVISION

GENERAL NOTES:

- |  |   |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
|--|---|---|------------------------|--------------------------|------------------|--------------|--|------------------------|---------------------------------|-------|-------------|-------|------|------|-------------|
| <p><b>1. MATERIALS</b></p> <table border="0"> <tr> <td>STRUCTURAL STEEL PLATE</td> <td>ASTM DESCRIPTION<br/>A529 / A572 / A1011</td> </tr> <tr> <td>HOT ROLLED MILL SHAPES</td> <td>A36 / A529 / A572 / A500</td> </tr> <tr> <td>COLD FORM SHAPES</td> <td>A653 / A1011</td> </tr> </table> <p><b>2. A325 &amp; A490 BOLT TIGHTENING REQUIREMENTS</b></p> <p>BOLTED JOINTS SHALL BE CONNECTED AND INSPECTED IN ACCORDANCE WITH THE "RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" UNLESS NOTED OTHERWISE ON THE KBS ERECTION DRAWINGS, ALL A490 BOLTS SHALL BE PRE-TENSIONED AND ALL A325 BOLTS IN SECONDARY MEMBERS (PURLINS, GIRTS, FRAMED OPENINGS, ETC.), AND ALL FLANGE BRACES MUST BE SNUG TIGHT. PRIMARY FRAMING (RIGID FRAMING AND BRACING) MUST BE SNUG TIGHT EXCEPT AS FOLLOWS:</p> <ol style="list-style-type: none"> <li>BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5 TONS.</li> <li>BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS-REVERSALS ON THE CONNECTIONS THE ENGINEER-OF-RECORD FOR THE PROJECT SHOULD BE CONSULTED TO EVALUATE FOR THIS CONDITION.</li> <li>THE PROJECT SITE IS LOCATED IN A HIGH SEISMIC AREA. FOR IBC BASED CODES, "HIGH SEISMIC AREA" IS DEFINED AS A SEISMIC DESIGN CATEGORY OF D, E OR F.</li> <li>ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS A SLIP-CRITICAL CONNECTION MUST BE FREE OF PAINT, OIL OR OTHER MATERIALS THAT REDUCE FRICTION AT CONTACT SURFACES. GALVANIZED OR LIGHTLY RUSTED SURFACES ARE ACCEPTABLE.</li> </ol> <p><b>3. STRUCTURAL SHOP COAT PAINT</b></p> <p>THE COAT OF SHOP PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING FOR ONLY A SHORT PERIOD OF EXPOSURE TO ATMOSPHERIC CONDITIONS. SHOP COAT PRIMER DOES NOT PROVIDE THE APPEARANCE, DURABILITY AND/OR PROTECTION OF AN APPROPRIATE FIELD APPLIED FINISH. KIRBY STANDARD SHOP COAT PAINT SHALL MEET OR EXCEED THE REQUIREMENTS OF FEDERAL SPECIFICATION TTP-636.</p> <p><b>4. TEMPORARY PANEL STORAGE</b></p> <p>PAINTED BUILDING PANELS WITH FLUOROPOLYMER FINISH ARE HIGH-QUALITY CONSTRUCTION MATERIALS. WHILE IN THE BUNDLE, PANELS SHOULD BE PROTECTED FROM HIGH TEMPERATURE, HUMIDITY AND MOISTURE. OTHERWISE, DAMAGE CAN OCCUR TO THE PAINTED SURFACE OF THE PANEL. PLEASE REFER TO THE "WARNING LABEL" THAT KIRBY APPLIES TO EACH BUNDLE OF FLUOROPOLYMER FINISHED PANELS FOR PROPER STORAGE PROCEDURES.</p> <p><b>5. TEMPORARY BRACING</b></p> <p>BUILDER/CUSTOMER SHALL SPECIFICALLY NOTE THAT BRACING FURNISHED BY KIRBY IS INTENDED TO BE USED FOR THE COMPLETED BUILDING; KIRBY DOES NOT REPRESENT OR GUARANTEE THAT THE BRACING WILL BE ADEQUATE AS TEMPORARY BRACING DURING ERECTION OF THE BUILDING.</p> <p><b>6. PANEL HANDLING</b></p> <p>METAL BUILDING PANELS ARE WAXED OR OILED FOR FINISH PROTECTION DURING SHIPPING AND STORAGE. THE WAX OR OIL MAKES THE PANELS SLIPPERY AND HAZARDOUS TO WALK ON OR STAND ON. THE WAX OR OIL CAN BUILD UP ON SHOES, GLOVES, AND CLOTHING MAKING CLIMBING OR WALKING ON OTHER COMPONENTS HAZARDOUS.</p> <p><b>7. ERECTION NOTES</b></p> <p>THE BUILDING MUST BE ERECTED ACCORDING TO THE FRAMING PLANS, STANDARD DETAILS, SPECIAL DETAILS, AND NOTES TO ASSURE COMPLIANCE WITH DESIGN LOADS AND BUILDING CODE REQUIREMENTS. FIELD MODIFICATION OF THE BUILDINGS OR BUILDING COMPONENTS WHICH WILL AFFECT THE STRUCTURAL INTEGRITY OF THE BUILDING WILL NOT BE ALLOWED WITHOUT PRIOR APPROVAL BY AN AUTHORIZED REPRESENTATIVE OF KIRBY BUILDING SYSTEMS.</p> <p><b>8. WELDING SPECIFICATIONS</b></p> <p>ALL SHOP WELDS ON MATERIALS GREATER THAN OR EQUAL TO 0.125" IN THICKNESS WERE PRODUCED IN ACCORDANCE WITH THE 2010 AWS D1.1 STRUCTURAL WELDING CODE - STEEL. THE REMAINING WELDS ON OTHER THINNER MATERIALS WERE PRODUCED IN ACCORDANCE WITH THE 2008 AWS D1.3 STRUCTURAL WELDING CODE - SHEET STEEL. ALL WELDING WAS PERFORMED BY AWS CERTIFIED WELDERS.</p> <p><b>9. BUILDING MAINTENANCE MANUAL</b></p> <p>AVAILABLE AT <a href="http://www.kirbybuildingsystems.com/for_metal_building_systems_builders.asp">http://www.kirbybuildingsystems.com/for_metal_building_systems_builders.asp</a></p> | STRUCTURAL STEEL PLATE                  | ASTM DESCRIPTION<br>A529 / A572 / A1011 | HOT ROLLED MILL SHAPES | A36 / A529 / A572 / A500 | COLD FORM SHAPES | A653 / A1011 | <p><b>1. MATERIALS</b></p> <table border="0"> <tr> <td>ROOF AND WALL SHEETING</td> <td>ASTM DESCRIPTION<br/>A653 / A792</td> </tr> <tr> <td>BOLTS</td> <td>A307 / A325</td> </tr> <tr> <td>CABLE</td> <td>A475</td> </tr> <tr> <td>RODS</td> <td>A572 / A108</td> </tr> </table> | ROOF AND WALL SHEETING | ASTM DESCRIPTION<br>A653 / A792 | BOLTS | A307 / A325 | CABLE | A475 | RODS | A572 / A108 |
| STRUCTURAL STEEL PLATE   | ASTM DESCRIPTION<br>A529 / A572 / A1011 |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
| HOT ROLLED MILL SHAPES   | A36 / A529 / A572 / A500                |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
| COLD FORM SHAPES   | A653 / A1011                            |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
| ROOF AND WALL SHEETING   | ASTM DESCRIPTION<br>A653 / A792         |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
| BOLTS  | A307 / A325                             |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
| CABLE  | A475                                    |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |
| RODS   | A572 / A108                             |   |                        |                          |                  |              |  |                        |                                 |       |             |       |      |      |             |

BUILDING LOADS / DESCRIPTION:

CERTIFICATION EXTENDS ONLY FOR THE LOADS SPECIFIED ON KIRBY'S PURCHASE ORDER TO THE STRUCTURAL COMPONENTS OF THE BUILDING DESIGNED AND SUPPLIED BY KIRBY BUILDING SYSTEMS, INC., IF ERECTED AS INDICATED. NOTE THAT KIRBY'S ENGINEER IS NOT ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT. DESIGN LOADS HAVE BEEN APPLIED IN ACCORDANCE WITH THE FOLLOWING.

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY: IBC 18

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD: 2.500 PSF (ROOF PANELS & PURLINS)

RISK CATEGORY: 1 - Low

COLLATERAL LOAD: 3.000 PSF

GROUND SNOW LOAD: 30.000 PSF Is: 0.800 Ct: 1.200 Ce: 1.000

ROOF SNOW LOAD: 30.000 PSF MINIMUM SNOW LOAD: 30.000 PSF

RAIN ON SNOW SURCHARGE: N/A PSF RAIN W/ SNOW (IF REQ'D): N/A PSF

ROOF LIVE LOAD: 40.000 PSF TRIBUTARY REDUCTION: No

FRAME LIVE LOAD: 40.000 PSF

ULTIMATE WIND SPEED  $V_{ult}$ : 115 MPH EXPOSURE: C lw: 1.000 KZT: 1.000 GCpt: +/- 0.18

NOMINAL WIND SPEED  $V_{red}$ : 89 MPH COMPONENTS & CLADDING: + 22 PSF, - 31 PSF

SEISMIC CRITERIA: Ss: 0.180 S1: 0.060 SDS: 0.171 SD1: 0.080

SEISMIC USE GROUP: N/A SITE CLASS: D lc: 1.000

SEISMIC DESIGN CATEGORY: B

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

STRUCTURAL SYSTEM: NOT DETAILED FOR SEISMIC

LATERAL DIRECTION - BASE SHEAR: 2.300 KIPS R: 3.000 CS: 0.057

LONGITUDINAL DIRECTION - BASE SHEAR: 1.410 KIPS R: 3.000 CS: 0.057

THIS BUILDING IS DESIGNED AS AN ENCLOSED STRUCTURE. ALL EXTERIOR COMPONENTS (DOORS, WINDOWS, ETC.) MUST BE DESIGNED TO WITHSTAND THE WIND LOADINGS SPECIFIED FOR THE DESIGN OF COMPONENTS AND CLADDING IN THE DESIGN CODE LISTED ABOVE. ALL EXTERIOR COMPONENTS (WINDOWS, DOORS, ETC) MUST MEET WIND LOADING REQUIREMENTS FOR THE BUILDING CODE LISTED ABOVE OR MUST BE ADEQUATELY PROTECTED DURING A HIGH WIND EVENT. ALL GLAZING AND OTHER APPLICABLE OPENINGS IN WINDBORNE DEBRIS REGIONS MUST BE IMPACT-RESISTANT OR PROTECTED WITH AN IMPACT-RESISTANT COVERING. IMPACT RESISTANT MATERIALS MUST MEET THE LARGE AND/OR SMALL MISSILE TEST OF ASTM E 1996 AND ASTM E 1886.

THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

HEAD LOAD: NORMAL WEIGHT OF METAL BUILDING COMPONENTS AS SUPPLIED BY THE MANUFACTURER

MINIMUM SNOW LOAD IS BASED ON THE MINIMUM ROOF SNOW LOAD CALCULATED PER BUILDING CODE OR THE CONTRACT-SPECIFIED ROOF SNOW LOAD, WHICHEVER IS GREATER. THIS VALUE FOR THE MINIMUM SNOW LOAD IS ONLY APPLIED IN COMBINATION WITH DEAD AND COLLATERAL LOADS. ROOF SNOW IN OTHER LOADING CONDITIONS IS DETERMINED PER THE SPECIFIC BUILDING CODE.

FOR OCCUPANCY CATEGORY I OR II BUILDINGS, IBC ALLOWS FOR SINGLE STORY BUILDINGS TO HAVE NO LIMIT FOR SEISMIC STORY DRIFT. PLEASE NOTE THAT ANY INTERIOR WALLS, PARTITIONS, CEILINGS, AND EXTERIOR WALLS SHOULD BE DETAILED (BY OTHERS) TO ACCOMMODATE THIS STORY DRIFT.

OTHER LOADS:

ENGINEER NOTES:

PROFESSIONAL CERTIFICATION: I HAROLD W. GREGORY, HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 20695, EXPIRATION DATE: 1/6/2021

PRIMER:

STRUCTURAL FRAMING: GP - GRAY PRIMER

WALL SECONDARY: GP - GRAY PRIMER

ROOF SECONDARY: GP - GRAY PRIMER

ROOF PANELS:

TYPE: 24 Ga. STANDING SEAM 360 (SS3)

LOW SYSTEM, NO THERMAL SPACERS

COLOR: TRK

NOTE: ANY VARIANCE FROM THE PANEL TYPES OR COLORS LISTED HERE WILL BE NOTED ON THE ELEVATION DRAWINGS.

TRIM COLORS:

ROOF LINE TRIM: TRK

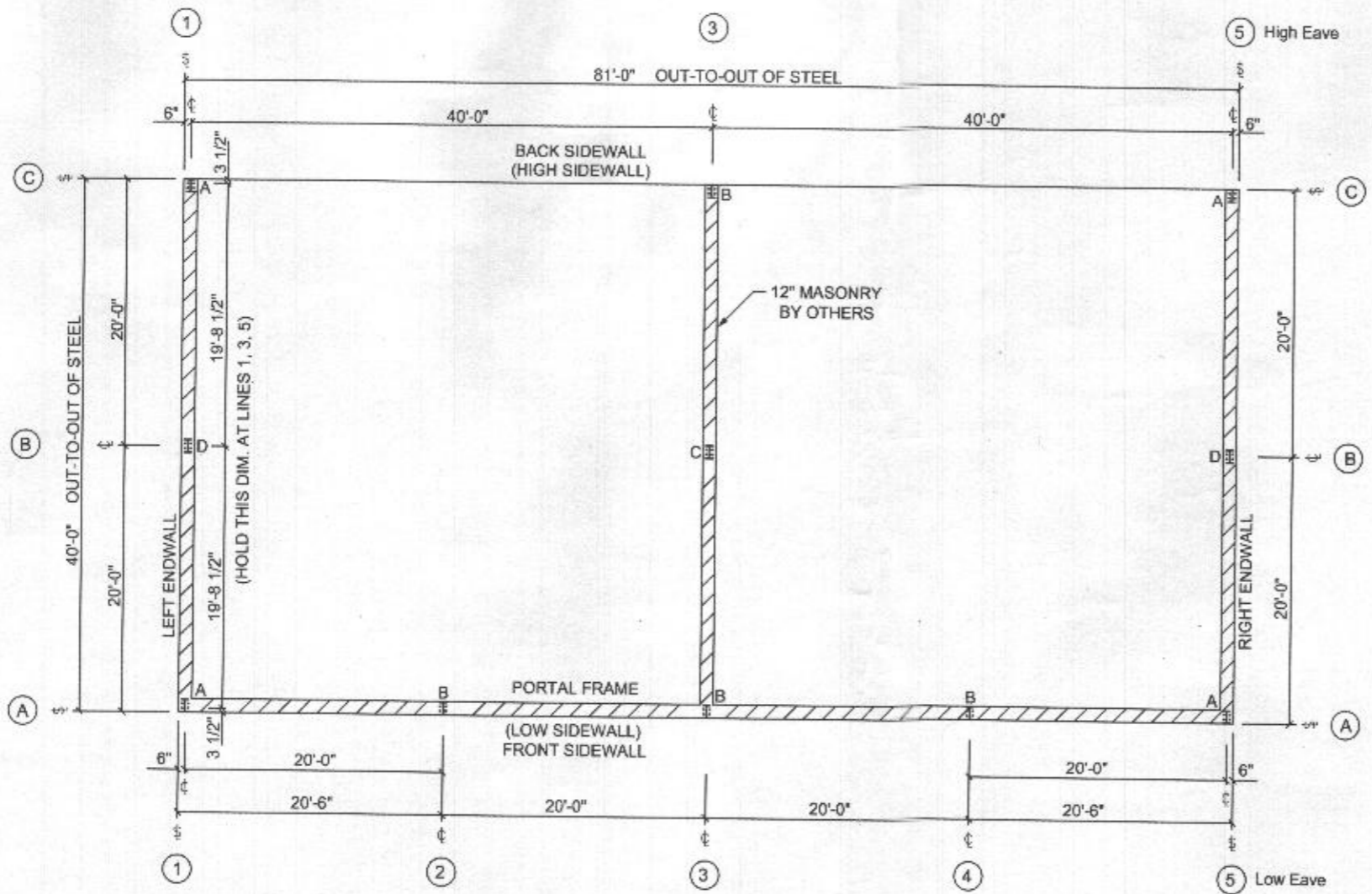
DOWNSPOUTS: TBS

PERMIT DRAWINGS

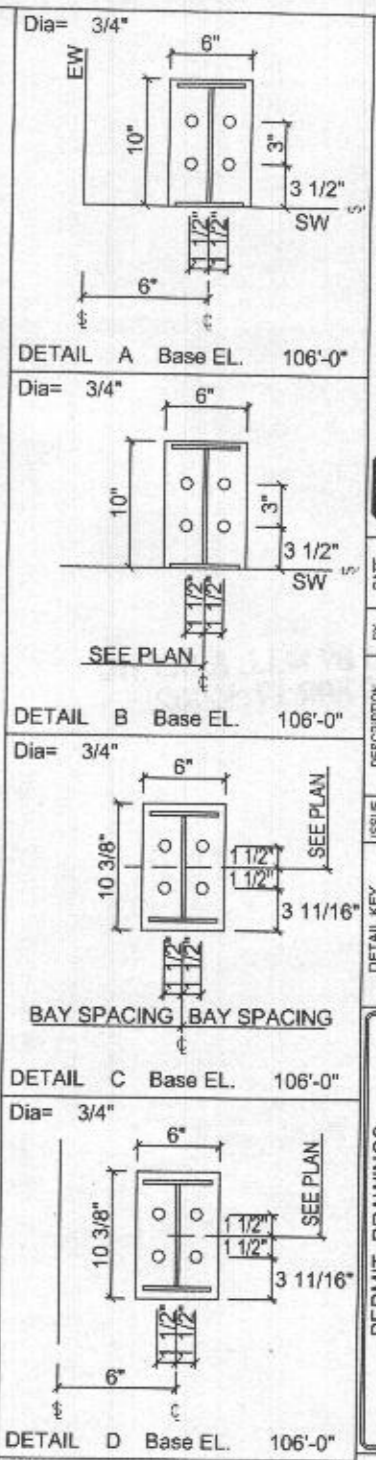
THESE DRAWINGS ACCURATELY DEPICT THE FINAL DESIGN OF THIS PROJECT AND MAY BE USED TO VERIFY THE SCOPE OF WORK. PRIOR TO THE DELIVERY OF THIS PROJECT, A SET OF DRAWINGS WILL BE ISSUED WITH THE PIECE-MARKS AND ADDITIONAL DETAIL NECESSARY FOR THE CONSTRUCTION OF THIS METAL BUILDING SYSTEM. NOTE: THESE DRAWINGS ARE NOT TO BE USED AS APPROVAL DRAWINGS AND ANY CHANGES MADE MAY DELAY THE DELIVERY OF THIS PROJECT.



NOTE: CONSTRUCTION BY OTHERS IS NOT SHOWN ON ANCHOR BOLT DETAILS FOR DRAWING CLARITY.



**ANCHOR BOLT PLAN**  
NOTE: ALL BASE PLATES @ 106'-0" (U.N.)  
FINISHED FLOOR @ 100'-0"



**IAS**  
124 KIRBY DR., PORTLAND, TN  
615-325-4165

**MBMA**  
MEMBER

**KIRBY BUILDING SYSTEMS**  
ANUCOR Company

124 KIRBY DR., PORTLAND, TN  
615-325-4165

BUYER: MTD DIRECTORS  
PROJECT: ALPHA RIDGE - ORGANS STORAGE STRUCTURE  
LOCATION: COLUMBIA, MD 21046

FINAL BY: [ ] DATE: [ ]  
CHK BY: [ ] DATE: [ ]  
JOB NO: K1900654A  
DWG NO: E1 01-8

DATE	BY	DESCRIPTION	ISSUE
10/24/19	BAL	APPROVAL	A
10/29/19	ASD	APPROVAL	A1
2/18/20	ASD	APPROVAL	A2

DETAIL KEY  
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DETAIL NAME: E1-ASG001  
IF NO PAGE IS CALLED OUT, SEE PAGES AT END OF DRAWING SET.

**PERMIT DRAWINGS**

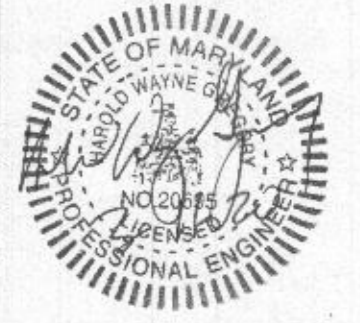
THESE DRAWINGS ACCURATELY DEPICT THE FINAL DESIGN OF THIS PROJECT AND MAY BE USED TO VERIFY THE SCOPE OF WORK. PRIOR TO THE DELIVERY OF THIS PROJECT, A SET OF DRAWINGS WILL BE ISSUED WITH THE PRE-MARKS AND ADDITIONAL DETAIL NECESSARY FOR THE CONSTRUCTION OF THIS METAL BUILDING SYSTEM. NOTE: THESE DRAWINGS ARE NOT TO BE USED AS APPROVAL DRAWINGS AND ANY CHANGES MADE MAY DELAY THE DELIVERY OF THIS PROJECT.

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A.B. SIZE AND PROJECTION	ANCHOR BOLT QUANTITY	PROJECTION (*P)
AS REQ'D	44	N/A
1-1/2"		3-1/2"
1-1/2"		3-1/2"

BOLT MATERIAL IS TO BE ASTM F1554-3B  
\*\* ANCHOR BOLT EMBEDMENT LENGTH 10" IS TO BE DETERMINED BY THE FOUNDATION ENGINEER.



**GENERAL NOTES:**

A81. ALL DIMENSIONS ARE OUT TO OUT OF STEEL. IF CONCRETE NOTCHES REQUIRED, THEN THE REED DIMENSIONS SHOULD BE ADDED TO OBTAIN THE OUT TO OUT OF CONCRETE DIMENSIONS.

A82. THIS DRAWING INDICATES WHERE THE ANCHOR BOLTS ARE TO BE PLACED AS WELL AS THE FOOTPRINT OF THE METAL BUILDING. IT IS ESSENTIAL THAT THESE BOLT PATTERNS BE FOLLOWED. IF THESE SETTINGS DIFFER FROM THE ARCHITECTURAL FOUNDATION PLANS, THE METAL BUILDING MANUFACTURER MUST BE CONTACTED IMMEDIATELY - BEFORE CONCRETE IS PLACED.

A83. EACH SPECIFIED ANCHOR BOLT DIAMETER ASSUMES F1554 GRADE 36 UNLESS NOTED OTHERWISE. ANCHOR BOLTS OF EQUAL DIAMETER THAT EXCEED THE F1554-36 STRENGTH REQUIREMENT MAY BE USED AT THE DISCRETION OF THE FOUNDATION ENGINEER.

A84. PROJECT FOUNDATION DESIGN SHALL BE THE RESPONSIBILITY OF A REGISTERED PROFESSIONAL ENGINEER FAMILIAR WITH LOCAL SITE CONDITIONS, NOT THE METAL BUILDING MANUFACTURER.

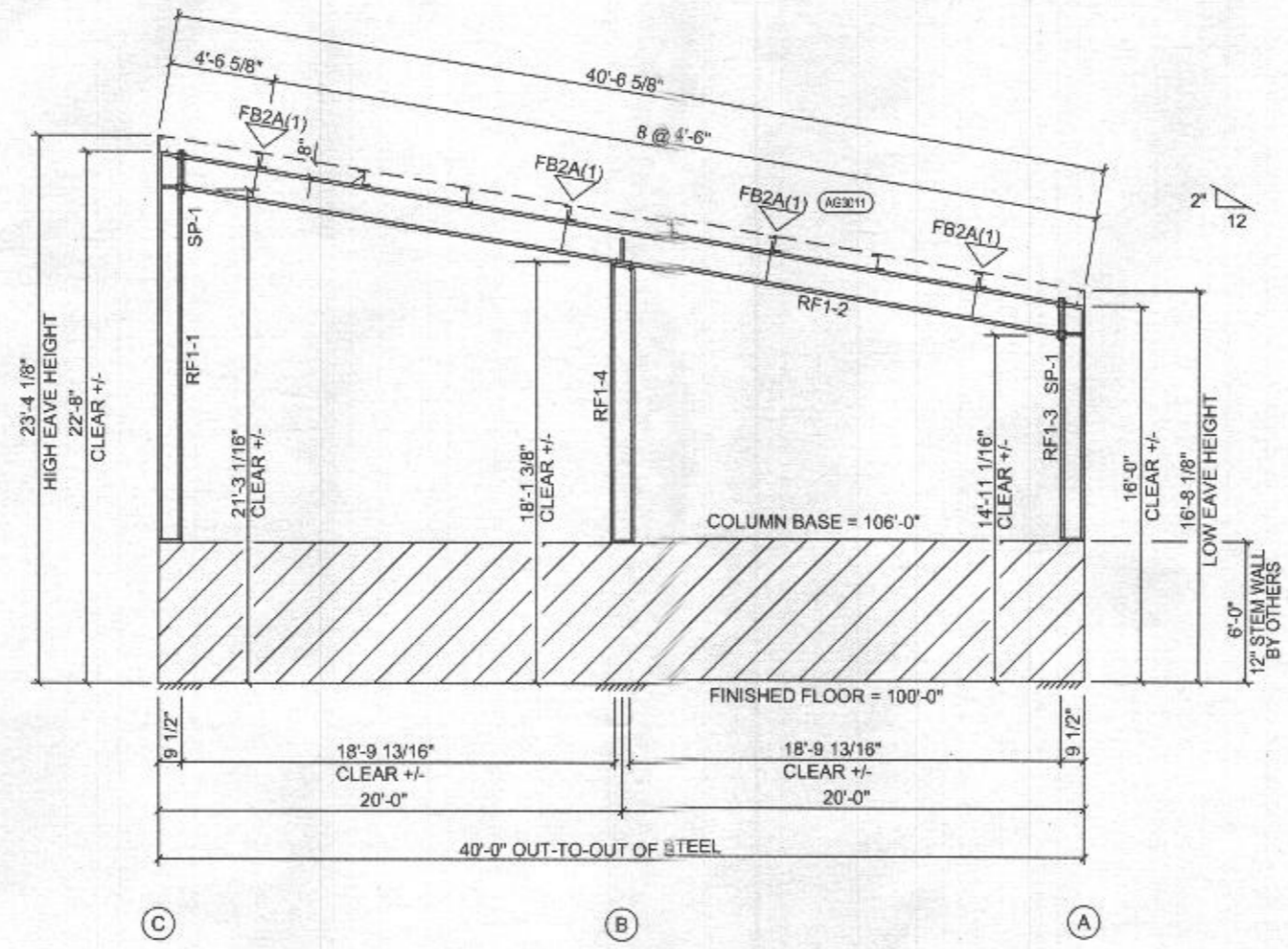
A85. THE METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR ANCHOR BOLTS (INCLUDING WASHERS AND NUTS), EXPANSION BOLTS, EMBEDMENT PLATES, NOR ANY OTHER MATERIALS THAT MAY BE CONSIDERED PART OF THE FOUNDATION.

A86. THE ANCHOR BOLT LOCATIONS SHOWN SATISFY PERTINENT REQUIREMENTS OF THE DESIGN OF THE METAL BUILDING. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO MAKE CERTAIN THAT SUFFICIENT CONCRETE EDGE DISTANCE IS PROVIDED FOR ALL ANCHOR BOLTS.

A87. DRAWINGS ARE NOT TO SCALE. SEE DETAILS FOR COLUMN ORIENTATION.

SPlice PLATE & BOLT TABLE										CAP PLATE BOLTS				
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length	Width	Thick	Length	Mark	Qty	Type	Dia	Length
SP-1	4	4	0	A325	3/4"	3"	6"	3/8"	1'-9 7/16"	RF1-4	4	A325	5/8"	2 1/4"

▽ FLANGE BRACES: (1) One Side; (2) Two Sides  
 FBxxA(1)  
 A - L2525105



RIGID FRAME ELEVATION: FRAME LINE 1 5

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QUALIFICATION EXTENDS TO THE STRUCTURAL COMPONENTS OF THE BUILDING DESIGNED AND SUPPLIED BY KIRBY BUILDING SYSTEMS. IF OTHERS ARE INDICATED, NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT.

APPLIED TO THE STRUCTURAL COMPONENTS OF THE BUILDING DESIGNED AND SUPPLIED BY KIRBY BUILDING SYSTEMS. IF OTHERS ARE INDICATED, NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT.

ISSUE	DESCRIPTION	BY	DATE
A	APPROVAL	BAL	10/24/19
A1	APPROVAL	ASD	10/20
A2	APPROVAL	ASD	2/15/20
P	PERMIT	ASD	3/4/20

DETAIL KEY  
 EXAMPLE PAGE CALLOUT  
 DETAIL NAME AG0081  
 IF NO PAGE IS CALLED OUT, SEE D-PAGES AT END OF DRAWING SET.

BUYER: MTD ERECTORS  
 PROJECT: ALPHA RIDGE - ORDNANCE STORAGE STRUCTURE  
 LOCATION: COLUMBIA, MD 21046  
 JOB NO: K'190265A  
 DWG NO: E2 OF 8

DATE: 3/4/20  
 DATE: 3/4/20  
 DATE: 3/4/20

CROSS SECTION: LINES 1, 5  
 DATE: 3/4/20  
 DATE: 3/4/20

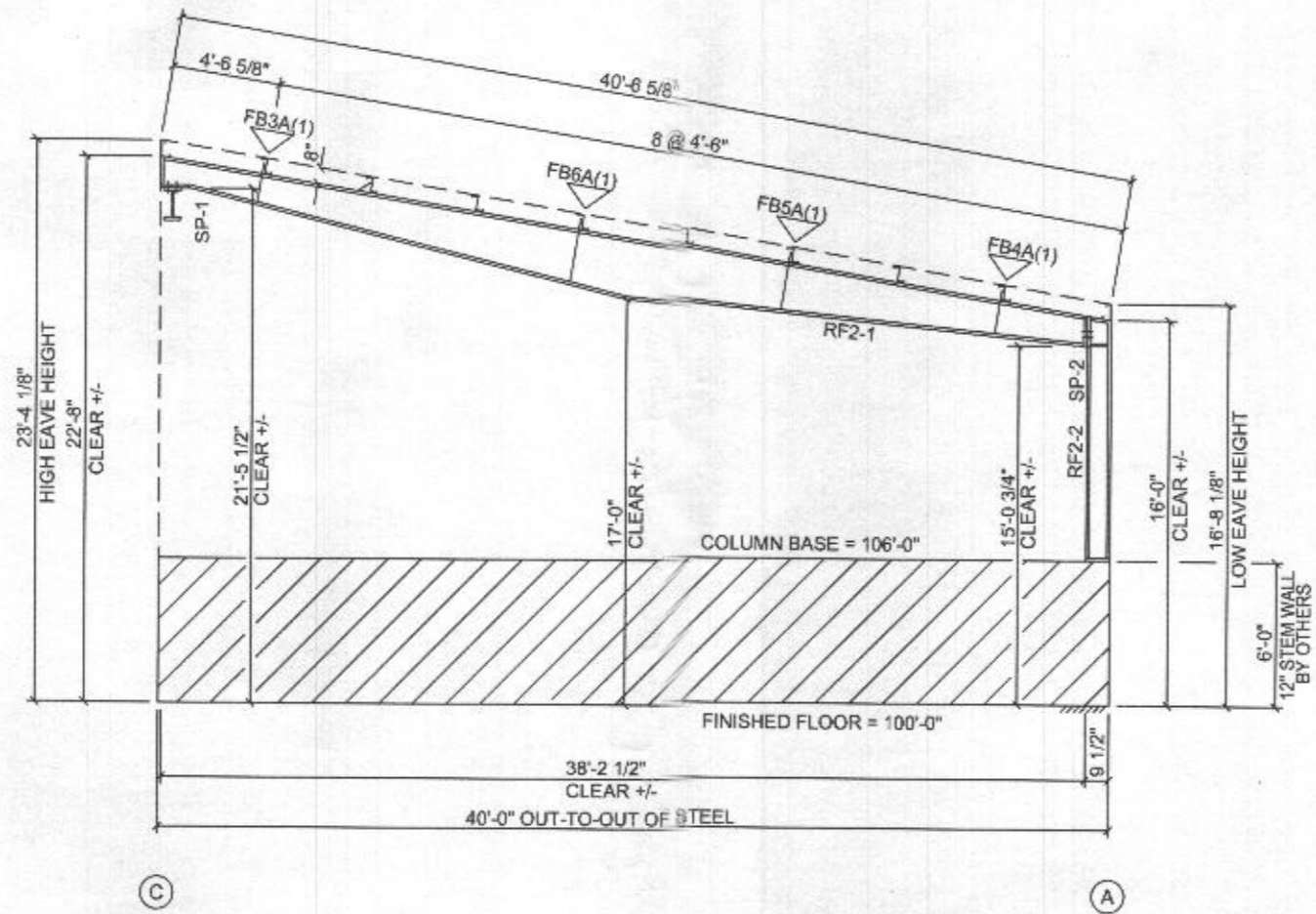
124 KIRBY DR, PORTLAND, TN  
 615-325-4165  
 MEMFA  
 A MFCOR Company

IAS  
 124 KIRBY DR, PORTLAND, TN  
 615-325-4165

SPLICE PLATE & BOLT TABLE

Mark	Qty		Int	Type	Dia	Length	Width	Thick	Length
	Top	Bot							
SP-1	4	0	0	A325	3/4"	3"	6"	3/8"	1'-0"
SP-2	4	0	0	A325	3/4"	3"	6"	3/8"	1'-1 15/16"

▽ FLANGE BRACES: (1) One Side; (2) Two Sides  
 FBxxA(1)  
 A - L2525105



RIGID FRAME ELEVATION: FRAME LINE 2 4

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**IAS**  
 124 KIRBY DR, PORTLAND, TN  
 615-325-4185

**KIRBY BUILDING SYSTEMS**  
 A HUBCOR Company

**MBMA**  
 METAL BUILDING MANUFACTURERS ASSOCIATION

BUYER: MTD ERECTORS  
 PROJECT: ALUM. REGE. -OHANWIS STORAGE STRUCTURE  
 LOCATION: COLUMBIA, MD 21046  
 JOB NO.: K1900654A  
 DWG. NO.: E3 CF 8

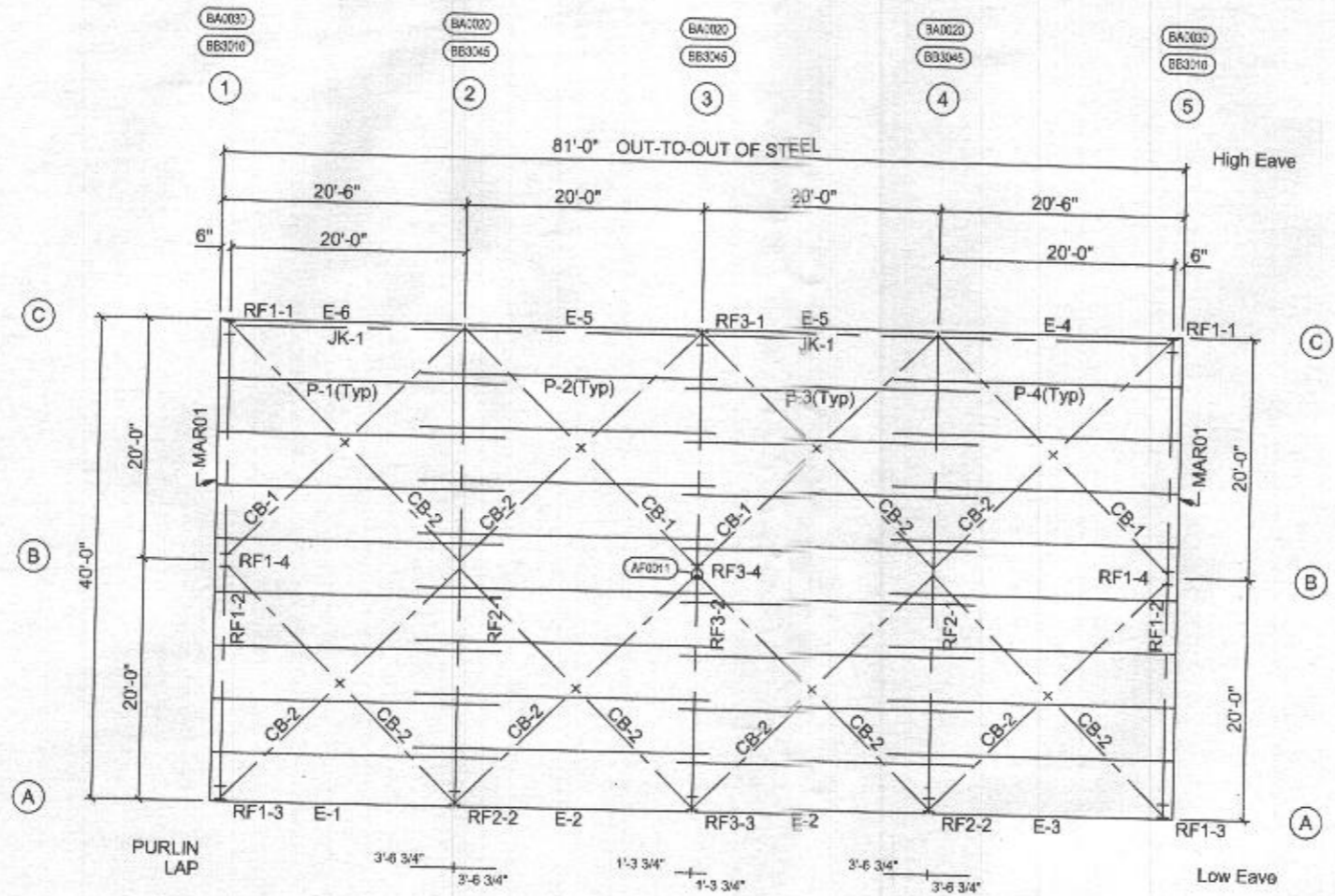
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2/15/20	ASD	APPROVAL	A2	DETAIL NAME - A40081
3/4/20	ASD	PERMIT	P	IF NO PAGE IS CALLED OUT, SEE CHANGES AT END OF DRAWING SET.

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ROOF FRAMING PLAN

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CERTIFICATION EXTENDS ONLY FOR THE LOADS SPECIFIED ON THESE PERMITS. PERMITS ARE NOT VALID FOR STRUCTURAL COMPONENTS OF BUILDING SYSTEMS, IF ERECTED AS SUPPLIED BY VARIOUS BUILDING SYSTEMS. NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT.

DETAIL KEY

EXAMPLE	PAGE	CALLOUT
E7	AG0081	

IF NO PAGE IS CALLED OUT, SEE D-PAGES AT END OF DRAWING SET.

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A1	APPROVAL	ASD	1/19/20
A2	APPROVAL	ASD	2/18/20
P	PERMIT	ASD	3/4/20

**KIRBY BUILDING SYSTEMS**  
 A FLUOR CORP. Company  
 124 KIRBY DR. PORTLAND, TN  
 615-325-4165

**MEMA**  
 MEMBERSHIP IN THE METAL BUILDING INDUSTRY ASSOCIATION

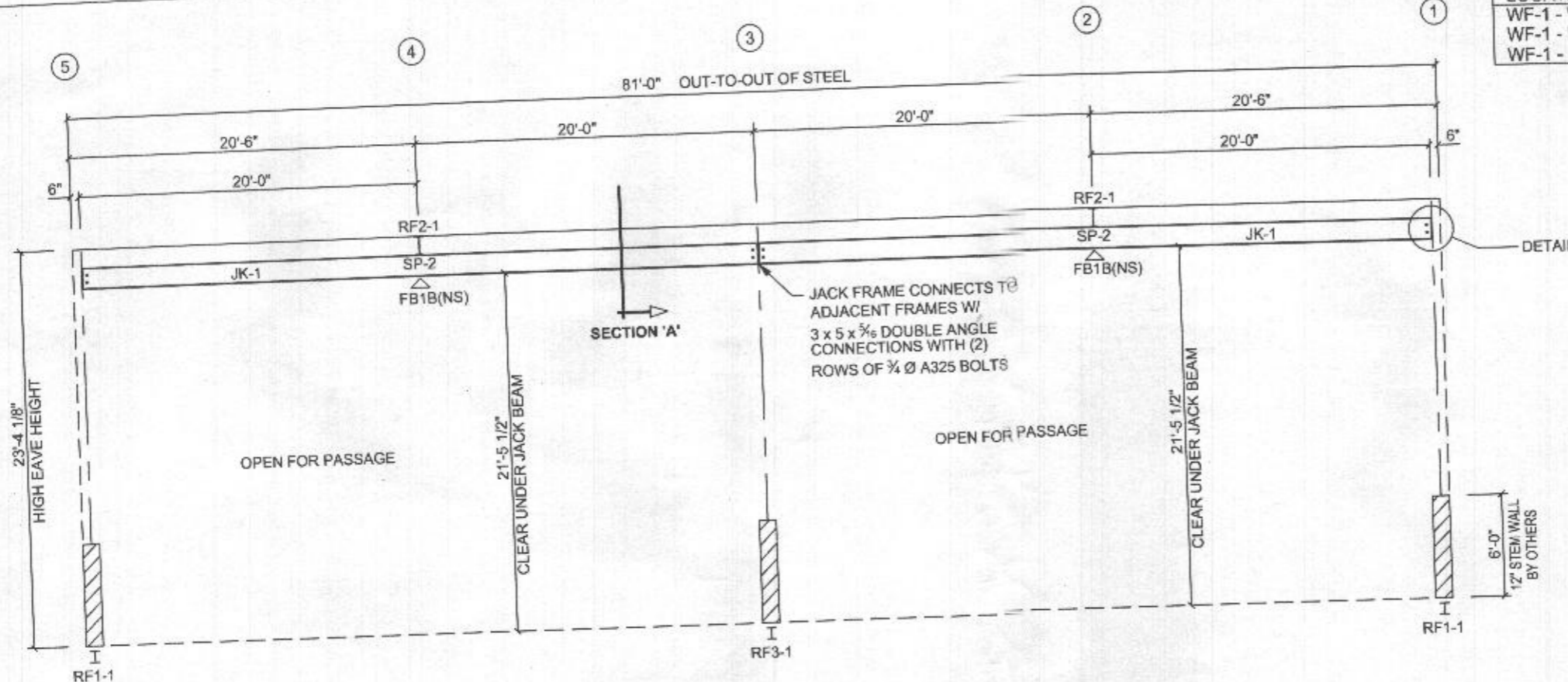
**IAS**  
 CONSULTING ENGINEERS

TITLE: ROOF FRAMING PLAN  
 BUYER: LTD ERectors  
 PROJECT: ALPHA-RIDGE ORGANICS STORAGE STRUCTURE  
 LOCATION: COLUMBIA, MD 21046

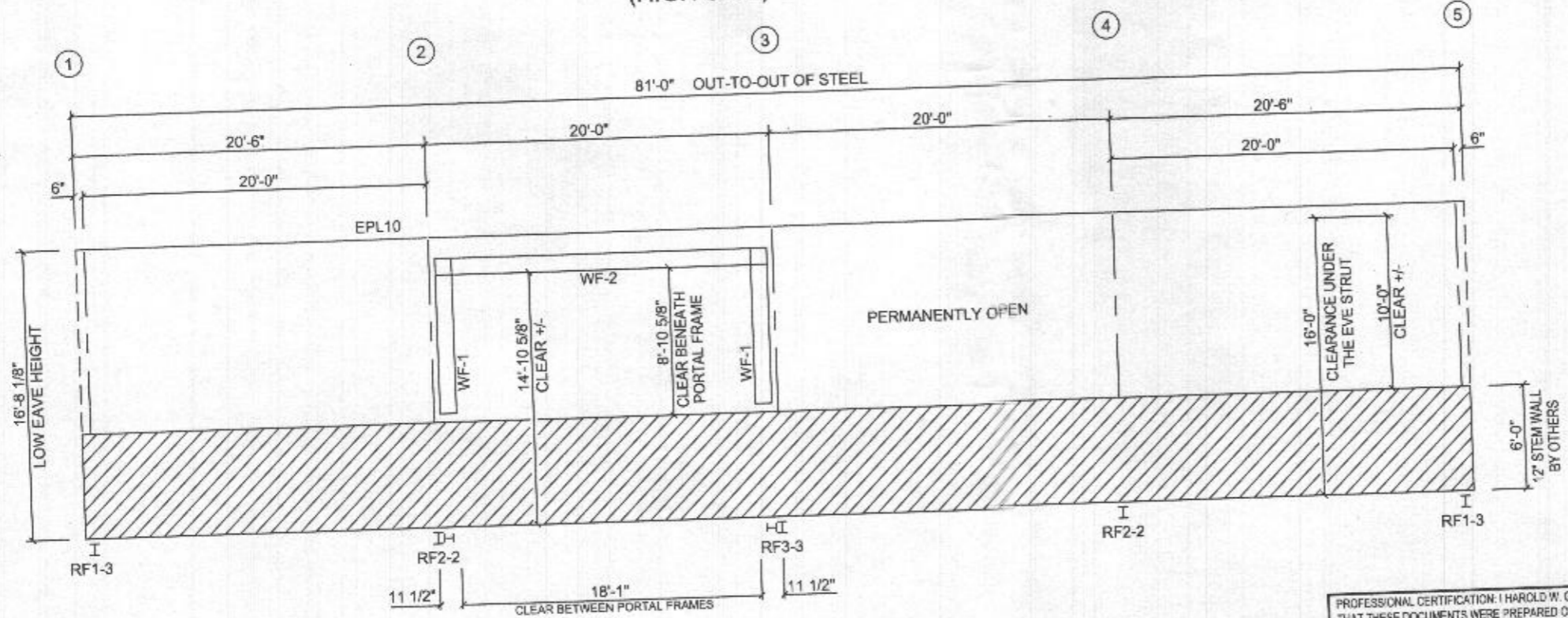
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FINAL BY: DATE: 3/4/20  
 CKD BY: DATE: 3/4/20  
 DWG NO: E5 OF 8





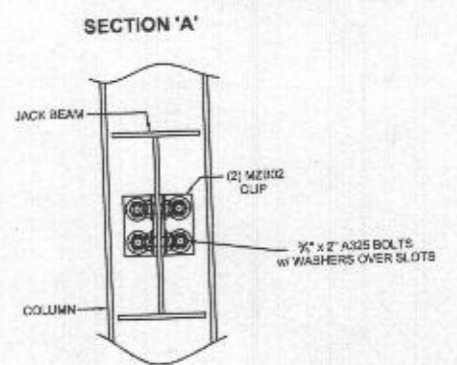
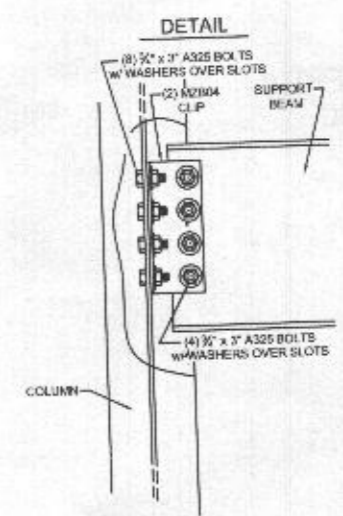
BACK SIDEWALL FRAMING: FRAME LINE C  
(HIGH SIDE)



FRONT SIDEWALL FRAMING: FRAME LINE A  
(LOW SIDE)

BOLT TABLE FRAME LINE C				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	3"
WF-1 - RF2-2	6	A325	1/2"	2"
WF-1 - RF3-3	6	A325	1/2"	2"

▽ FLANGE BRACES: Both Sides(U.N.)  
FBxxA(1)  
B - L3X3X1/4



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DIVISION

PROFESSIONAL CERTIFICATION: I HAROLD W. GREGORY, HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 20685, EXPIRATION DATE: 1/5/2021.



DATE		BY		DESCRIPTION		ISSUE		DETAIL KEY	
10/24/19	BAL	ASD	ASD	APPROVAL	A	CALL OUT	EXAMPLE PAGE	ET	AG008
1/9/20	ASD	ASD	ASD	APPROVAL	A1				
2/18/20	ASD	ASD	ASD	APPROVAL	A2				
3/4/20	ASD	ASD	ASD	PERMIT	P				

IF NO PAGE IS CALLED OUT SEE L-PAGES AT END OF DRAWING SET.

**PERMIT DRAWINGS**

THESE DRAWINGS ACCURATELY DEPICT THE FINAL DESIGN OF THIS PROJECT AND MAY BE USED TO VERIFY THE SCOPE OF WORK PRIOR TO THE DELIVERY OF THIS PROJECT. A SET OF DRAWINGS WILL BE ISSUED WITH THE PERMIT MARKS AND ADDITIONAL DETAIL NECESSARY FOR THE CONSTRUCTION OF THIS METAL BUILDING SYSTEM. NOTE: THESE DRAWINGS ARE NOT TO BE USED AS APPROVAL DRAWINGS AND ANY CHANGES MADE MAY DELAY THE DELIVERY OF THIS PROJECT.

CERTIFICATION EXTENDS ONLY FOR THE LOADS SPECIFIED ON PERMITS PURCHASE UNDER THIS ACT. THE ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF THE BUILDING OR THE SYSTEMS, IF ERECTED AS INDICATED. NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE UNDERWRITER OF CONSTRUCTION PROJECT.

**IAS**  
124 KIRBY DR, PORTLAND, TN  
615-325-4165

**KIRBY BUILDING SYSTEMS**  
A Nucor Company

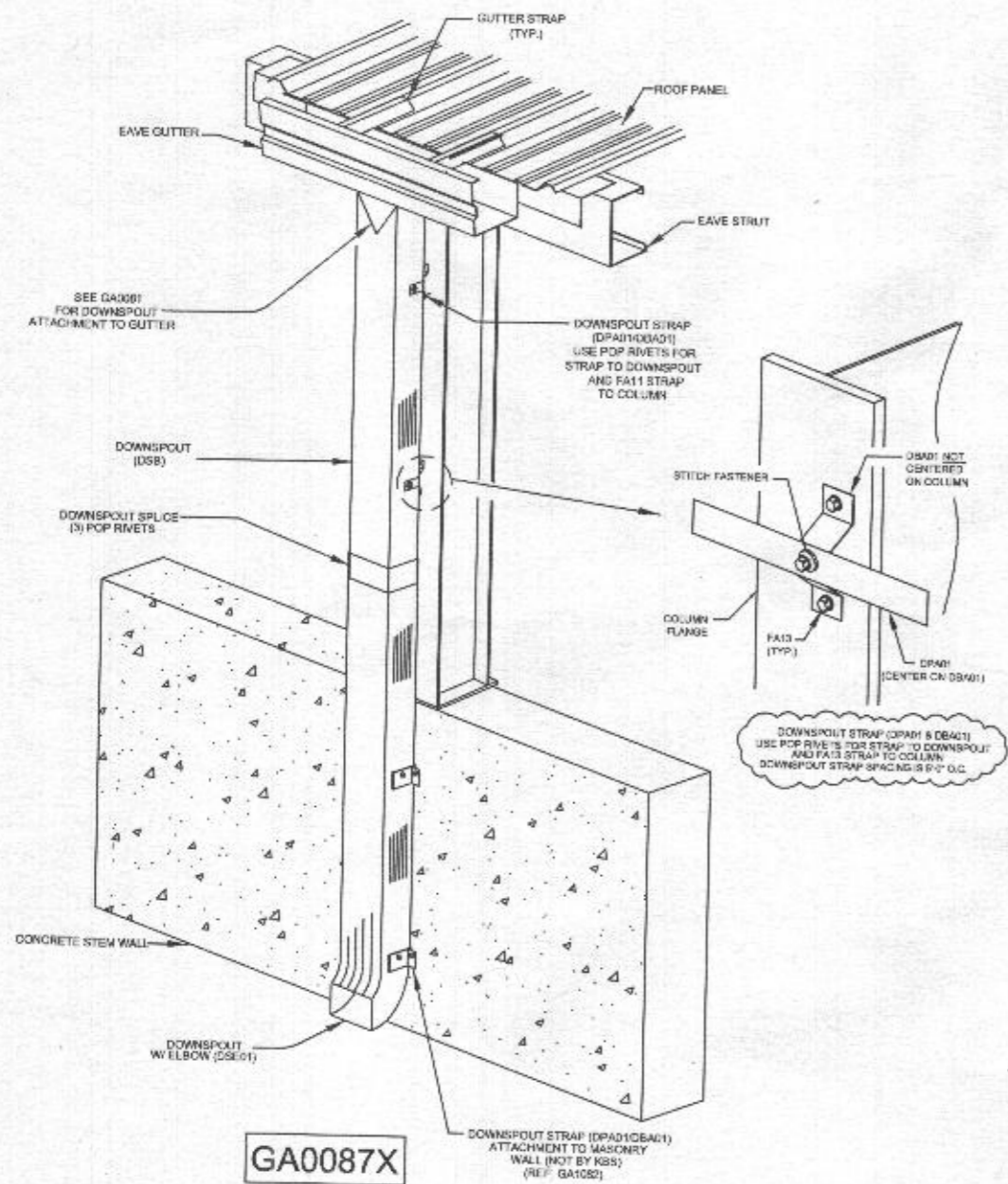
**MBMA**  
MEMBER

DATE: \_\_\_\_\_  
BY: \_\_\_\_\_  
TITLE: SIDEWALL FRAMING: LINES A, B

DATE: \_\_\_\_\_  
BY: \_\_\_\_\_  
TITLE: ALPHABETIC ORGANICS STORAGE STRUCTURE

DATE: \_\_\_\_\_  
BY: \_\_\_\_\_  
TITLE: \_\_\_\_\_

DWG NO: ET 8  
CR: 8



GA0087X

**IAS**  
REGISTERED ARCHITECT

**KIRBY BUILDING SYSTEMS**  
MEMPHIS  
AN URS CORP. COMPANY

124 KIRBY DR. PORTLAND, TN  
615-325-4165

BUYER: MTD ERECTORS  
PROJECT: ALPHA RIDGE ORNAMS STORAGE STRUCTURE  
LOCATION: COLUMBIA, MD 21046  
JOB NO: K18J0054A  
DWG NO: E8 OF 8

ISSUE	DESCRIPTION	BY	DATE
A	APPROVAL	BAL	10/24/19
A'	APPROVAL	ASD	1/8/20
A2	APPROVAL	ASD	2/18/20
P	PERMIT	ASD	3/4/20

DETAIL KEY  
 EXAMPLE PAGE CALLOUT  
 DETAIL NAME E7  
 NAME AG0087

IF NO PAGE IS CALLED OUT, SEE D-PAGES AT END OF DRAWING SET.

**PERMIT DRAWINGS**

THESE DRAWINGS ACCURATELY DEPICT THE FINAL DESIGN OF THIS PROJECT AND MAY BE USED TO VERIFY THE SCOPE OF WORK. PRIOR TO THE DELIVERY OF THIS PROJECT, A SET OF DRAWINGS WILL BE ISSUED WITH THE PERMITS AND ADDITIONAL DETAIL NECESSARY FOR THE CONSTRUCTION OF THIS METAL BUILDING SYSTEM. NOTE: THESE DRAWINGS ARE NOT TO BE USED AS APPROVAL DRAWINGS AND ANY CHANGES MADE MAY DELAY THE DELIVERY OF THIS PROJECT.

CERTIFICATION EXTENDS ONLY FOR THE LOADS SPECIFIED ON KIRBY'S PERMITS. THE STRUCTURAL COMPONENTS OF THE BUILDING DESCRIBED AND SUPPLIED BY KIRBY BUILDING SYSTEMS, IF ERRECTED AS INDICATED, NOTE THAT KIRBY'S ENGINEER IS NOT ACTING AS THE ENGINEER OF RECORD FOR THIS CONSTRUCTION PROJECT.

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